



DRAINAGE REPORT

BMR JAMAICA WIND LIMITED MUNRO EXPANSION PROJECT SAINT ELIZABETH, JAMAICA



34 SCHOOL STREET • LITTLETON, NH 03561 • PHONE 603-444-4111 • FAX 603-444-1343 • www.horizonsengineering.com

DRAINAGE REPORT FOR BMP JAMAICA WIND LIMITED MUNRO EXPANSION PROJECT SAINT ELIZABETH, JAMAICA

JANUARY 2014

PROJECT NUMBER 13218 Copyright © 2014 Horizons Engineering, Inc.

17 Sunset Terrace Newport, VT 05855 Ph.: 802-334-6434 Fax: 802-334-5602 34 School Street Littleton, NH 03561 Ph: 603-444-4111 Fax: 603-444-1343 www.horizonsengineering.com 35 Railroad Row, Suite #204 White River Junction, VT 05001 Ph: 802-296-8300 Fax: 802-296-8301

TABLE OF CONTENTS

1.0 PROJECT INFORMATION NARRATIVE

- 1.1 Project Narrative
 - 1.1.1 Project Summary
 - 1.1.2 Existing Conditions
 - 1.1.2.1 Soils Information
 - 1.1.2.2 Site Photographs
 - 1.1.3 Proposed Site Conditions & Disturbances
 - 1.1.4 Rainfall Data
 - 1.1.5 Peak Runoff Summary

2.0 DRAINAGE CALCULATIONS, ANALYSIS & DESIGN

- 2.1 Pre-development Analysis
 - 2.1.1 Node listing for the 2, 10, and 50 storm events
 - 2.1.2 Full summary and Diagram for the 10 year storm event
- 2.2 Post-development Analysis
 - 2.2.1 Node listing for the 2, 10 and 50 storm events
 - 2.2.2 Full summary and Diagram for the 10 year storm event
- 2.3 References Preparer's/Reviewer's Certification

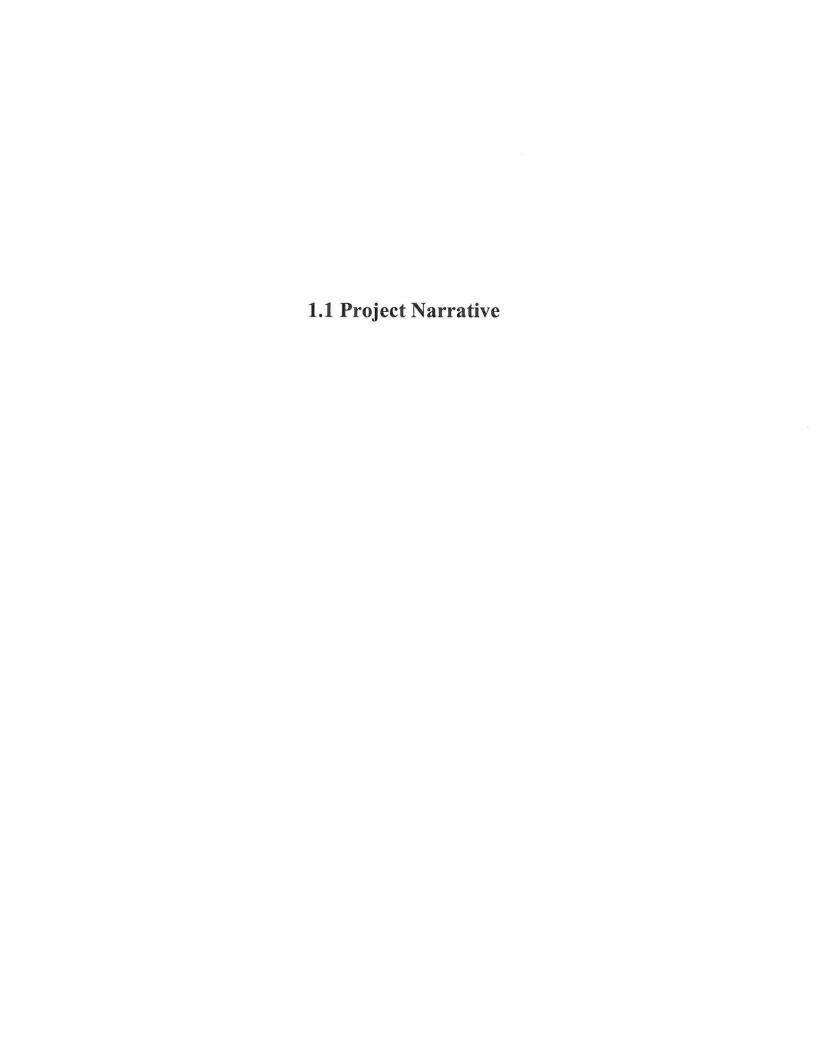
3.0 PLANS

- 3.1 Design Plans (unbound)
- 3.2 Pre and Post Development Drainage Area Plans

LIST OF TABLES

- Table 1.1 -Comparison of Type II and Type III Storms for 10, 50 Year Events (Pre-development)
- Table 1.2 Type II, 24 Hour Rainfall Depths for Project Site
- Table 1.3 2, 10, and 50 Year Comparison
- Table 2.1 Soil Types and Drainage Description

1.0 PROJECT INFORMATION NARRATIVE

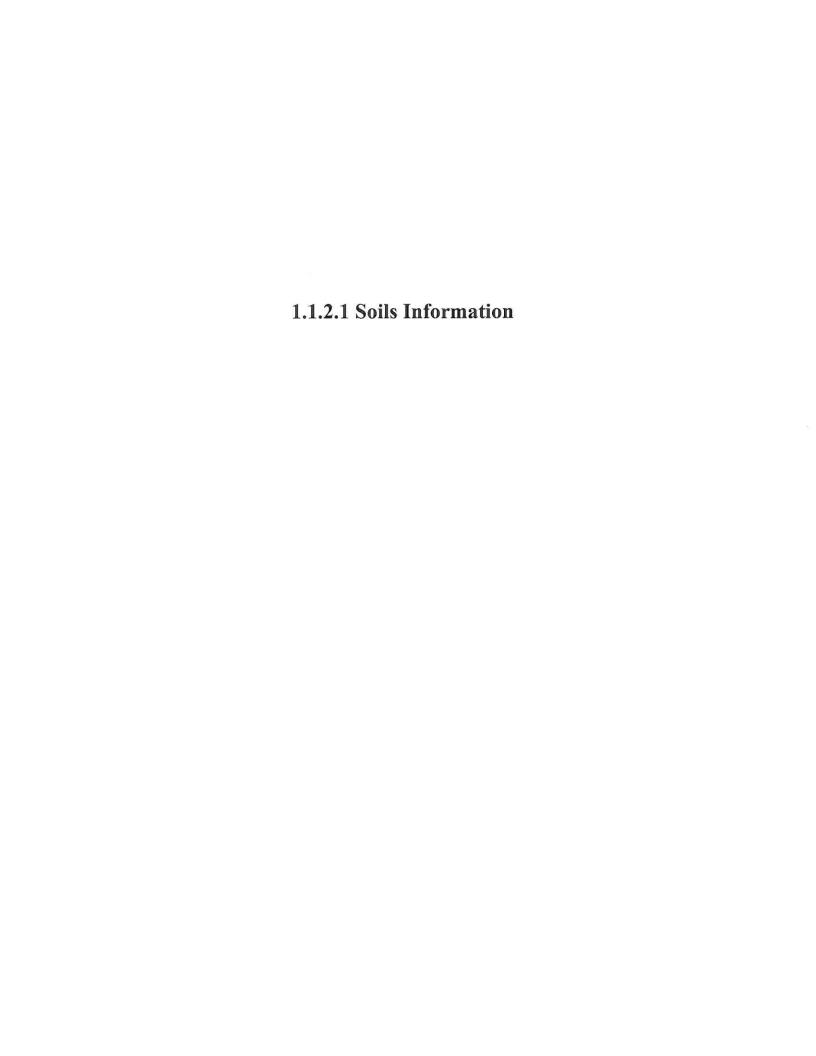


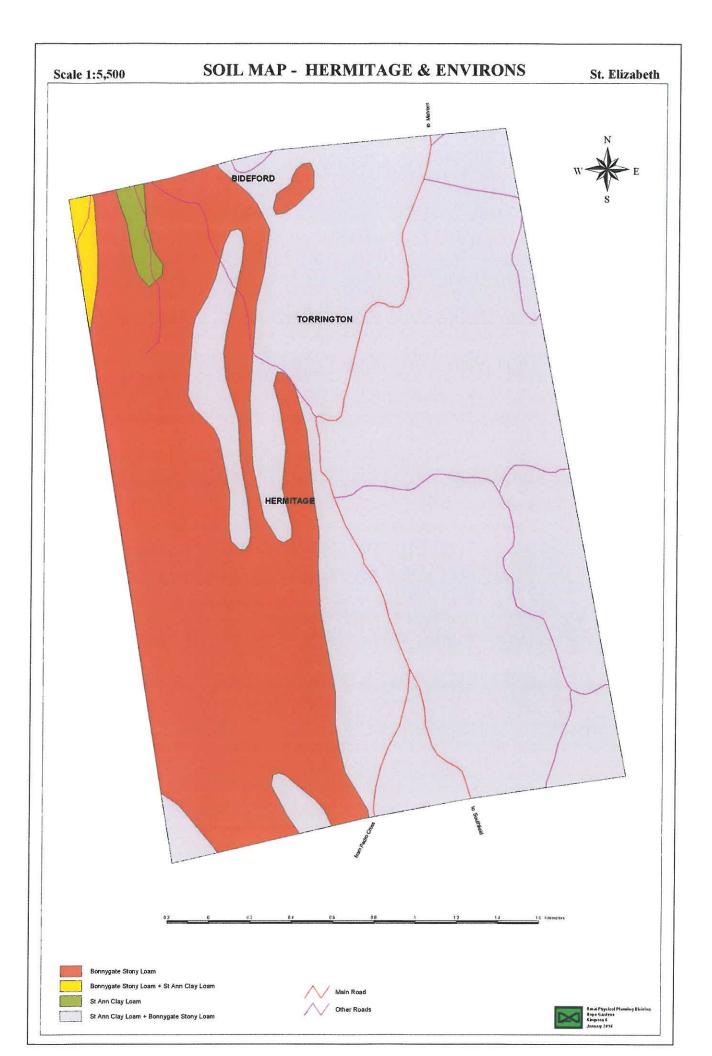
1.1.1 Project Summary

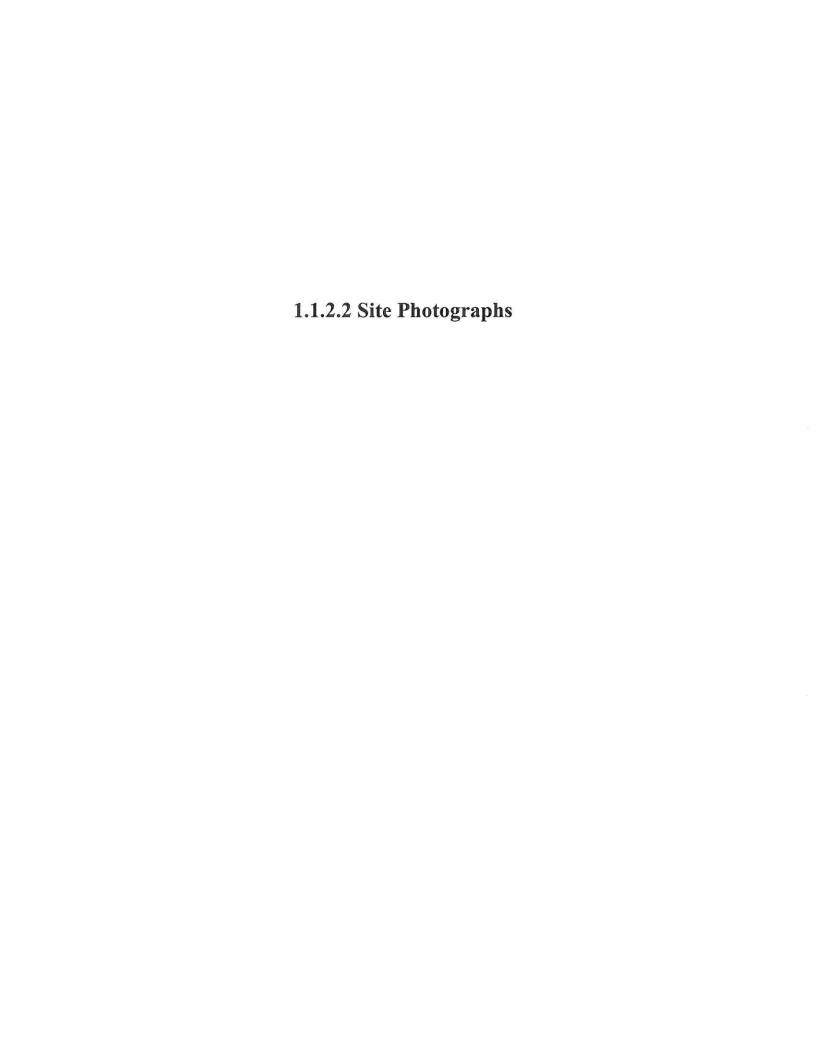
BMR Jamaica Wind Limited has been selected to expand the Munro Wind Project in Saint Elizabeth, Jamaica. The site currently has four turbines located in a north-south string. The proposal is to take advantage of an excellent wind resource and expand the project to 18 new Vestas turbines located along four new access roads, totaling approximately 4,170 meters of roadway. Additionally a laydown area, an operations and maintenance building, an electrical substation, electrical transmission mains (other buried and overhead) are proposed.

1.1.2 Existing Site Conditions

The Munro Wind Project expansion is located on the edge of a plateau near Potsdam, Saint Elizabeth. The elevation in the area of the project is relatively level at around 700 meters above sea level. The project site is rural in character with a mix of forested and grassed areas. There are a number of sinkholes, which can be attributed to the karstic nature of the underlying Newport formation limestone. To the west the forested terrain falls away quickly to an elevation of around 45 meters at an average slope between 30% and 50%.







The following photos are included to provide a general overview of existing conditions on the

project site.



Looking southeast toward existing turbines #1 and #2 from near existing turbine #3



Photo characteristic of onsite vegetation and topography, near proposed turbine #14



Looking northward at existing turbines #1 - #4 from near proposed turbine #17



Looking southwest over forested slope



Looking south at existing turbines #1 and #2, from near proposed substation

1.1.3 Proposed Site Conditions & Disturbances

Approximately 248,985 square meters of earth disturbance will be required for construction of turbine pads, roads, and other infrastructure on the site. This work will include installing an electrical substation, operations and maintenance building, temporary material laydown pad, and electrical transmission lines (both above and below ground). The design intent is that culverts and ditches be sized for the 25 year storm event, and detention structures have at least 15cm freeboard during the 50 year event.

The impacts to water quality during site development will be minimized using temporary treatment devices and erosion control measures. Frequent site inspections during construction are required during or directly following rainfall events to ensure erosion control devices are working properly.

1.1.4 Rainfall Data

Using SCS TR-20, run under HydroCAD Version 9.1, pre- and post-development cover types and drainage paths were modeled to generate peak discharge rates. Type II and Type III-24 hour rainfall events were considered. Type II storms demonstrate intense short duration rainfall and were chosen for this project as they result in higher (more conservative) runoff rates...

Table 1.1 -Comparison of Type II and Type III Storms for 10, 50 Year Events (Predevelopment)

de (ciopinent)									
Watershed Area Discharge Point	Type II 10 Yr Flow Rate (cms)	Type III 10 Yr Flow Rate (cms)	Type II 50 Yr Flow Rate (cms)	Type III 50 Yr Flow Rate (cms)					
W	6.702	6.423	60.141	56.876					
X	0.251	0.233	2.775	2.416					
Y	0.353	0.336	3.026	2.813					
Z	0.549	0.559	4.293	4.191					

Depth of rainfall for various return periods was provided by the National Meteorological Service, Jamaica from a rain gage located in Potsdam, St. Elizabeth, approximately 2.5 kilometers from the center of the project site. These data are provided below in **Table 1.2**.

Table 1.2 - Type II, 24 Hour Rainfall Depths for Potsdam, St. Elizabeth

Rainfall Event	Depth*		
2-Year	84.6 mm		
5-Year	158.1 mm		
10-Year	228.1 mm		
25-Year	335.2 mm		
50-Year	425.5 mm		
100-Year	522.8 mm		

^{*}Data provided by Meteorological Service of Jamaica, Climate Branch

1.1.5 Peak Runoff Summary

Table 1.3 summarizes the stormwater runoff peak flow rate for the 2, 10 and 50 year storm events. With the exception of the 50 year storm at discharge point Z, all discharge points peak flow rates either decrease for the all storm events. The increase shown at discharge point Z for the 50 year event is approximately 3.4%. This increase does not take into account any infiltration of stormwater.

Table 1.3 - 2, 10 and 50 Year Comparison

	Pre 2 Yr	Post 2 Yr	Pre 10 Yr	Post 10 Yr	Pre 50 Yr	Post 50 Yr
Watershed Area	Flow Rate	Flow Rate	Flow Rate	Flow Rate	Flow Rate	Flow Rate
Discharge Point	(cms)	(cms)	(cms)	(cms)	(cms)	(cms)
W	0.000	0.000	6.702	6.645	60.141	60.367
X	0.000	0.000	0.251	0.250	2.775	2.765
Υ	0.000	0.000	0.353	0.334	3.026	2.864
Z	0.000	0.000	0.549	0.540	4.293	4.435

2.0 - DRAINAGE CALCULATIONS, ANALYSIS & DESIGN



2.1 Pre-development Analysis

Based on topography, four pre-development drainage areas have been identified on the project site to accommodate all areas disturbed by the project. These areas drain to points designated as W, X, Y, and Z throughout the drainage analysis and drainage report, as well as in the HydroCAD Model. The total watershed to be analyzed is 816.84 hectares.

Drainage area Pr1 is located on the west and south of the project site, draining west to discharge point W, a point where all flow off the western side of the project area is concentrated. Limits of available topography prevent this point from being precisely located for this study. This drainage area is primarily located on a steep, forested hillside. In the southeast of this drainage area is a portion of Munro College. Three of the four existing wind turbines (to be removed) are included in this area. This drainage area encompasses the majority of the project site, and is approximately 713.35 hectares.

Drainage area Pr2 is located in the northeast of the project site and encompasses a primarily open area with moderate hills. This area drains to discharge point X in the northeast of the drainage area. There is an existing road which runs along the top of the watershed on the west side. This drainage area is approximately 16.57 hectares.

Drainage area Pr3 is located in the center of the east of the project site, and drains to discharge point Y in the northeast of the drainage area. The area is gently sloping, and is primarily open. As with area Pr2, there are some existing roadways within this drainage area. One of the existing wind turbines (to be removed) is included in this drainage area. Pr3 is approximately 27.38 hectares.

Drainage area Pr4 is located in the center of the east of the project site, and drains to discharge point Z in the northeast of the drainage area. The area is hilly near the top of the watershed, and flatter near the bottom. The drainage area is a mix of open and forested areas. There are some existing roadways within this drainage area. This drainage area is approximately 59.53 hectares.

Information on soils in the area was obtained from the Rural Physical Planning Division of the Ministry of Agriculture. This information can be found in **Section 1.1.2.1** of this report. Soils on this site have been identified as Bonnygate Stony Loam, and a complex of St. Ann Clay Loam and Bonnygate Stony Loam. Soil types are described in the 'Soil Technical Guide Sheets' published by the Rural Physical Planning Division, 2004.

Table 2.1 – Soil Types and Drainage Description

Soil ID	Soil Name	Internal Drainage Description
77	Bonnygate	Very Rapid
78	St. Ann	Extremely Rapid

The 'Soil Technical Guide Sheets' advise that rapid internal drainage is defined by permeability rates of greater than 5.0 inches per hour. The United States Department of Agriculture Natural Resources Conservation Service defines Hydrologic Soil Group (HSG) A as soils that "have low runoff potential and high infiltration rates ... and have a high rate of water transmission (greater than 0.30 in/hr)". As such, all soils in the watershed have been interpreted to be HSG A. The watershed areas and drainage paths can be found in **Section 3.**

2.1.1 Pre-Development Node Listing 2, 10 and 50 - Year Storm Event

PRE DRAINAGE

Type II 24-hr 2-Year Rainfall=85 mm Printed 1/31/2014

Prepared by Horizons Engineering, Inc.

HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Page 1

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment Pr1: Drainage Area Runoff Area=7,133,532.7 m² 0.78% Impervious Runoff Depth=0 mm Flow Length=3,573.8 m Tc=118.8 min CN=31 Runoff=0.000 m³/s 0.000 MI

Subcatchment Pr2: Drainage Area Runoff Area=165,704.3 m² 0.81% Impervious Runoff Depth=0 mm Flow Length=581.1 m Tc=47.4 min CN=31 Runoff=0.000 m³/s 0.000 MI

Subcatchment Pr3: Drainage Area Runoff Area=273,843.1 m² 2.40% Impervious Runoff Depth=0 mm Flow Length=1,029.1 m Tc=91.9 min CN=32 Runoff=0.000 m³/s 0.000 MI

Subcatchment Pr4: Drainage Area Runoff Area=595,274.4 m² 2.32% Impervious Runoff Depth=0 mm Flow Length=2,751.8 m Tc=158.7 min CN=32 Runoff=0.000 m³/s 0.000 MI

Link W: Drainage Point Inflow=0.000 m³/s 0.000 MI
Primary=0.000 m³/s 0.000 MI

Link X: Drainage Point Inflow=0.000 m³/s 0.000 MI
Primary=0.000 m³/s 0.000 MI

Link Y: Drainage Point Inflow=0.000 m³/s 0.000 MI
Primary=0.000 m³/s 0.000 MI

Link Z: Drainage Point Inflow=0.000 m³/s 0.000 MI
Primary=0.000 m³/s 0.000 MI

Total Runoff Area = 816.8355 ha Runoff Volume = 0.000 Ml Average Runoff Depth = 0 mm 99.05% Pervious = 809.0954 ha 0.95% Impervious = 7.7401 ha

PRE DRAINAGE

Type II 24-hr 10-Year Rainfall=228 mm Printed 1/31/2014

Prepared by Horizons Engineering, Inc. HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Page 2

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment Pr1: Drainage Area Runoff Area=7,133,532.7 m² 0.78% Impervious Runoff Depth>14 mm Flow Length=3,573.8 m Tc=118.8 min CN=31 Runoff=6.702 m³/s 102.326 MI

Subcatchment Pr2: Drainage Area Runoff Area=165,704.3 m² 0.81% Impervious Runoff Depth>16 mm Flow Length=581.1 m Tc=47.4 min CN=31 Runoff=0.251 m³/s 2.570 MI

Subcatchment Pr3: Drainage Area Runoff Area=273,843.1 m² 2.40% Impervious Runoff Depth>17 mm
Flow Length=1,029.1 m Tc=91.9 min CN=32 Runoff=0.353 m³/s 4.626 MI

Subcatchment Pr4: Drainage Area Runoff Area=595,274.4 m² 2.32% Impervious Runoff Depth>16 mm Flow Length=2,751.8 m Tc=158.7 min CN=32 Runoff=0.549 m³/s 9.280 MI

Link W: Drainage PointInflow=6.702 m³/s 102.326 MI

Primary=6.702 m³/s 102.326 MI

Link X: Drainage Point Inflow=0.251 m³/s 2.570 MI
Primary=0.251 m³/s 2.570 MI

Link Y: Drainage Point Inflow=0.353 m³/s 4.626 MI Primary=0.353 m³/s 4.626 MI

Link Z: Drainage Point Inflow=0.549 m³/s 9.280 MI Primary=0.549 m³/s 9.280 MI

Total Runoff Area = 816.8355 ha Runoff Volume = 118.802 MI Average Runoff Depth = 15 mm 99.05% Pervious = 809.0954 ha 0.95% Impervious = 7.7401 ha

Prepared by Horizons Engineering, Inc. HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Page 3

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment Pr1: Drainage Area Runoff Area=7,133,532.7 m² 0.78% Impervious Runoff Depth>93 mm Flow Length=3,573.8 m Tc=118.8 min CN=31 Runoff=60.141 m³/s 662.847 MI

Subcatchment Pr2: Drainage Area Runoff Area=165,704.3 m² 0.81% Impervious Runoff Depth>97 mm Flow Length=581.1 m Tc=47.4 min CN=31 Runoff=2.775 m³/s 16.128 MI

Subcatchment Pr3: Drainage Area Runoff Area=273,843.1 m² 2.40% Impervious Runoff Depth>101 mm Flow Length=1,029.1 m Tc=91.9 min CN=32 Runoff=3.026 m³/s 27.537 MI

Subcatchment Pr4: Drainage Area Runoff Area=595,274.4 m² 2.32% Impervious Runoff Depth>96 mm Flow Length=2,751.8 m Tc=158.7 min CN=32 Runoff=4.293 m³/s 56.970 MI

Link W: Drainage PointInflow=60.141 m³/s 662.847 MI
Primary=60.141 m³/s 662.847 MI

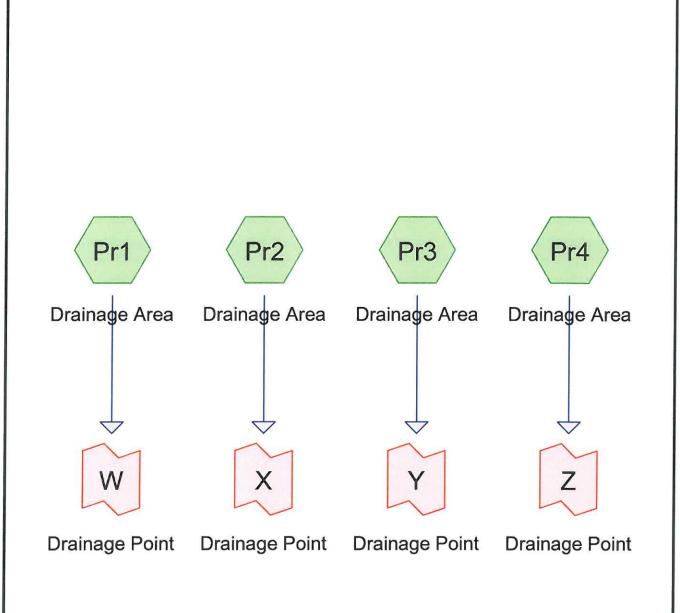
Link X: Drainage Point Inflow=2.775 m³/s 16.128 MI Primary=2.775 m³/s 16.128 MI

Link Y: Drainage Point Inflow=3.026 m³/s 27.537 MI Primary=3.026 m³/s 27.537 MI

Link Z: Drainage PointInflow=4.293 m³/s 56.970 MI
Primary=4.293 m³/s 56.970 MI

Total Runoff Area = 816.8355 ha Runoff Volume = 763.482 MI Average Runoff Depth = 93 mm 99.05% Pervious = 809.0954 ha 0.95% Impervious = 7.7401 ha

2.1.2 Pre-Development Full Summary and Diagram 10 - Year Storm Event











Prepared by Horizons Engineering, Inc., Printed 1/31/2014 HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

PRE_DRAINAGE
Prepared by Horizons Engineering, Inc.
HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Printed 1/31/2014 Page 2

Area Listing (all nodes)

Area CN		Description		
(hectares)		(subcatchment-numbers)		
242.6291	30	Meadow, non-grazed, HSG A (Pr1, Pr2, Pr3, Pr4)		
566.4663	30	Woods, Good, HSG A (Pr1, Pr2, Pr4)		
1.0845	98	xBUILDING (Pr1)		
6.6556	98	xROAD (Pr1, Pr2, Pr3, Pr4)		
816.8355	31	TOTAL AREA		

PRE_DRAINAGE
Prepared by Horizons Engineering, Inc.
HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Printed 1/31/2014 Page 3

Soil Listing (all nodes)

Area	Soil	Subcatchment
(hectares)	Group	Numbers
809.0954	HSG A	Pr1, Pr2, Pr3, Pr4
0.0000	HSG B	
0.0000	HSG C	
0.0000	HSG D	
7.7401	Other	Pr1, Pr2, Pr3, Pr4
816.8355		TOTAL AREA

Prepared by Horizons Engineering, Inc.
HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Page 4

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment Pr1: Drainage Area Runoff Area=7,133,532.7 m² 0.78% Impervious Runoff Depth>14 mm Flow Length=3,573.8 m Tc=118.8 min CN=31 Runoff=6.702 m³/s 102.326 MI

Subcatchment Pr2: Drainage Area Runoff Area=165,704.3 m² 0.81% Impervious Runoff Depth>16 mm Flow Length=581.1 m Tc=47.4 min CN=31 Runoff=0.251 m³/s 2.570 MI

Subcatchment Pr3: Drainage Area Runoff Area=273,843.1 m² 2.40% Impervious Runoff Depth>17 mm
Flow Length=1,029.1 m Tc=91.9 min CN=32 Runoff=0.353 m³/s 4.626 MI

Subcatchment Pr4: Drainage Area Runoff Area=595,274.4 m² 2.32% Impervious Runoff Depth>16 mm Flow Length=2,751.8 m Tc=158.7 min CN=32 Runoff=0.549 m³/s 9.280 MI

Link W: Drainage Point Inflow=6.702 m³/s 102.326 MI Primary=6.702 m³/s 102.326 MI

Link X: Drainage Point Inflow=0.251 m³/s 2.570 MI
Primary=0.251 m³/s 2.570 MI

Link Y: Drainage Point Inflow=0.353 m³/s 4.626 MI Primary=0.353 m³/s 4.626 MI

Link Z: Drainage Point Inflow=0.549 m³/s 9.280 MI Primary=0.549 m³/s 9.280 MI

Total Runoff Area = 816.8355 ha Runoff Volume = 118.802 Ml Average Runoff Depth = 15 mm 99.05% Pervious = 809.0954 ha 0.95% Impervious = 7.7401 ha

Page 5

Summary for Subcatchment Pr1: Drainage Area

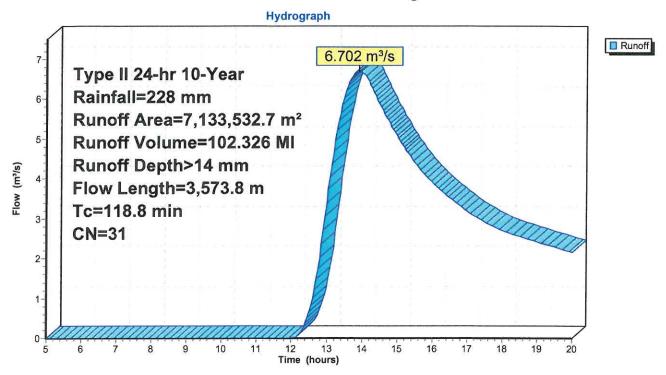
6.702 m³/s @ 13.93 hrs, Volume= 102.326 MI, Depth> 14 mm Runoff

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=228 mm

	Α	rea (m²)	CN D	escription		
	5,53	5,684.0	30 W	oods, Goo	d, HSG A	
•	' 4	4,796.9	98 xF	ROAD		
3	1	0,844.7	98 xE	BUILDING		
	1,54	2,207.1	30 M	eadow, no	n-grazed, F	HSG A
	7,13	3,532.7	31 W	eighted Av	/erage	
	7,07	7,891.1	99	22% Pen	ious Area	
	5	5,641.6	0.	78% Impe	rvious Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(meters)	(m/m)	(m/sec)	(m³/s)	
	28.6	31.0	0.0100	0.02		Sheet Flow, Pr1-A
						Grass: Bermuda n= 0.410 P2= 85 mm
	19.7	581.0	0.1048	0.49		Shallow Concentrated Flow, Pr1-B
						Woodland Kv= 1.52 m/s
	19.5	697.5	0.0781	0.60		Shallow Concentrated Flow, Pr1-C
						Short Grass Pasture Kv= 2.13 m/s
	11.6	767.9	0.5307	1.11		Shallow Concentrated Flow, Pr1-D
						Woodland Kv= 1.52 m/s
	7.2	355.3	0.1511	0.83		Shallow Concentrated Flow, Pr1-E
						Short Grass Pasture Kv= 2.13 m/s
	32.2	1,141.1	0.1514	0.59		Shallow Concentrated Flow, Pr1-F
_						Woodland Kv= 1.52 m/s
	118.8	3,573.8	Total			

Page 6

Subcatchment Pr1: Drainage Area



Summary for Subcatchment Pr2: Drainage Area

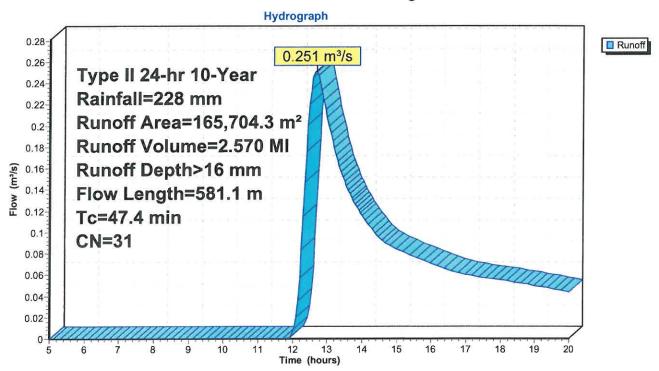
Runoff = 0.251 m³/s @ 12.69 hrs, Volume=

2.570 MI, Depth> 16 mm

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=228 mm

	Α	rea (m²)	CN I	Description		
	4	5,455.7	30 \	Voods, Goo	d, HSG A	
*		1,336.4	98	ROAD		
*		0.0	98	BUILDING		
	11	8,912.2	30 [leadow, no	n-grazed, H	ISG A
	16	5,704.3	31 \	Veighted Av	/erage	
	16	4,367.9	9	9.19% Pen	vious Area	
		1,336.4	(.81% Impe	rvious Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(meters)	(m/m	(m/sec)	(m³/s)	
	21.3	31.0	0.0200	0.02	308 87	Sheet Flow, Pr2-A
						Woods: Light underbrush n= 0.400 P2= 85 mm
	26.1	550.1	0.0273	0.35		Shallow Concentrated Flow, Pr2-B
						Short Grass Pasture Kv= 2.13 m/s
	47.4	581.1	Total			

Subcatchment Pr2: Drainage Area



Page 8

HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

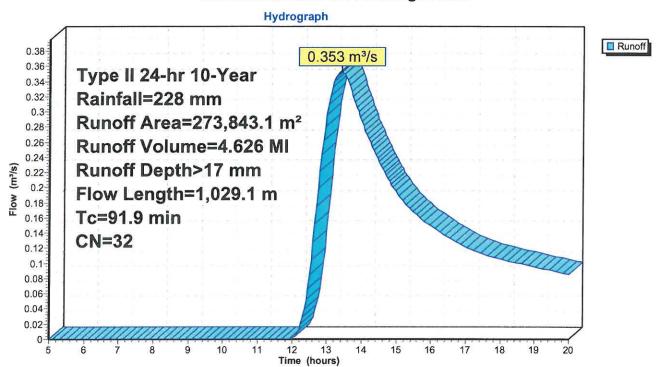
Summary for Subcatchment Pr3: Drainage Area

0.353 m³/s @ 13.41 hrs, Volume= 4.626 MI, Depth> 17 mm Runoff

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=228 mm

	Α	rea (m²)	CN	Description	ili and a second	
		0.0	30	Woods, God	od, HSG A	
*		6,584.9	98	xROAD		
*		0.0	98	xBUILDING		
	26	7,258.2	30	Meadow, no	n-grazed, F	HSG A
	27	3,843.1	32	Weighted Av	verage	
	26	7,258.2	3	97.60% Per	vious Area	
		6,584.9	10	2.40% Impe	rvious Area	
		S				
	Tc	Length	Slope	e Velocity	Capacity	Description
	(min)	(meters)	(m/m) (m/sec)	(m ³ /s)	·
	28.6	31.0	0.010	0.02	75.	Sheet Flow, Pr3-A
						Grass: Bermuda n= 0.410 P2= 85 mm
	63.3	998.1	0.015	2 0.26		Shallow Concentrated Flow, Pr3-B
						Short Grass Pasture Kv= 2.13 m/s
5700.55	91.9	1,029.1	Total			

Subcatchment Pr3: Drainage Area



Summary for Subcatchment Pr4: Drainage Area

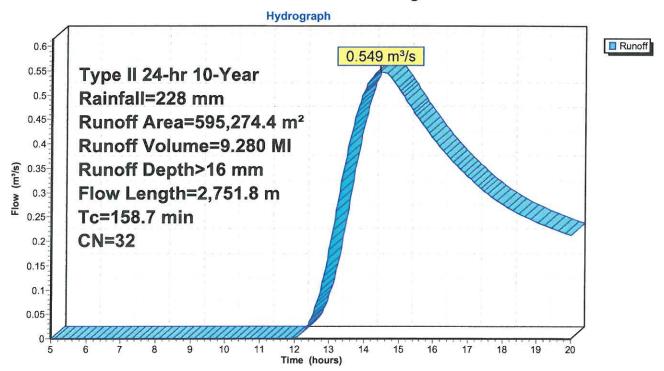
Runoff = 0.549 m³/s @ 14.47 hrs, Volume= 9.280 Ml, Depth> 16 mm

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=228 mm

	Α	rea (m²)	CN E	escription		
	8	33,523.1	30 V	Voods, Goo	od, HSG A	
1	٠ 1	3,837.6	98 x	ROAD		
	F	0.0	98 x	BUILDING		
-	49	7,913.7	30 N	leadow, no	n-grazed, F	HSG A
	59	5,274.4	32 V	Veighted Av	verage	
	58	1,436.8	9	7.68% Pen	vious Area	
	1	3,837.6	2	.32% Impe	rvious Area	
	Tc	Length	Slope	Velocity	Capacity	Description
100	(min)	(meters)	(m/m)	(m/sec)	(m^3/s)	
-	12.5	31.0	0.0757	0.04		Sheet Flow, Pr4-A
						Woods: Light underbrush n= 0.400 P2= 85 mm
	146.2	2,720.8	0.0212	0.31		Shallow Concentrated Flow, Pr4-B
		u-orona-constitution				Short Grass Pasture Kv= 2.13 m/s
-	1507	2 751 0	Total			

158.7 2,751.8 Total

Subcatchment Pr4: Drainage Area



Summary for Link W: Drainage Point

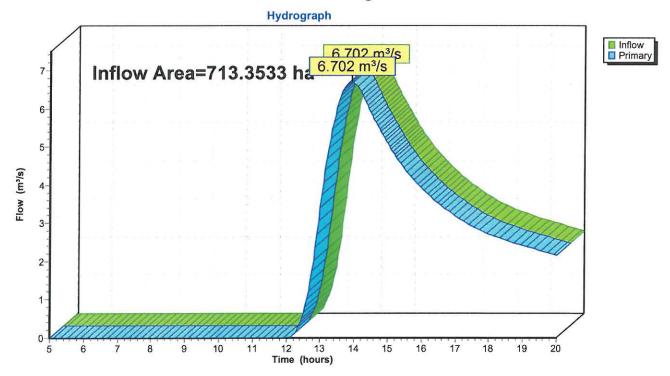
Inflow Area =

Inflow

713.3533 ha, 0.78% Impervious, Inflow Depth > 14 mm for 10-Year event $6.702 \text{ m}^3\text{/s}$ @ 13.93 hrs, Volume= 102.326 MI $6.702 \text{ m}^3\text{/s}$ @ 13.93 hrs, Volume= 102.326 MI, Atten= 0%, Lag= 0.0 mi 102.326 MI, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link W: Drainage Point



HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Page 11

Summary for Link X: Drainage Point

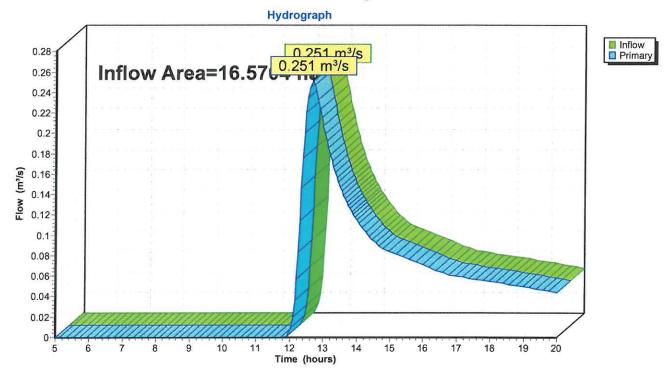
16.5704 ha, 0.81% Impervious, Inflow Depth > 16 mm for 10-Year event Inflow Area =

Inflow 2.570 MI

0.251 m³/s @ 12.69 hrs, Volume= 0.251 m³/s @ 12.69 hrs, Volume= 2.570 MI, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link X: Drainage Point



Summary for Link Y: Drainage Point

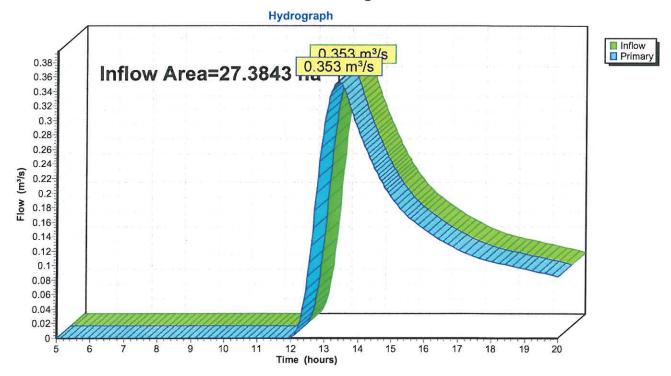
Inflow Area = 27.3843 ha, 2.40% Impervious, Inflow Depth > 17 mm for 10-Year event

0.353 m³/s @ 13.41 hrs, Volume= Inflow 4.626 MI

0.353 m³/s @ 13.41 hrs, Volume= Primary 4.626 MI, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link Y: Drainage Point



Printed 1/31/2014

Page 13

Summary for Link Z: Drainage Point

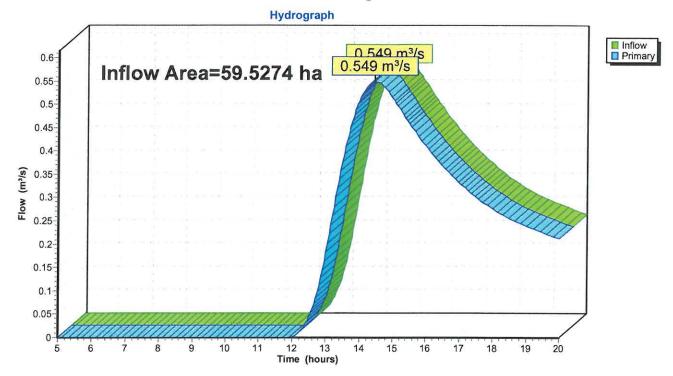
Inflow Area = 59.5274 ha, 2.32% Impervious, Inflow Depth > 16 mm for 10-Year event

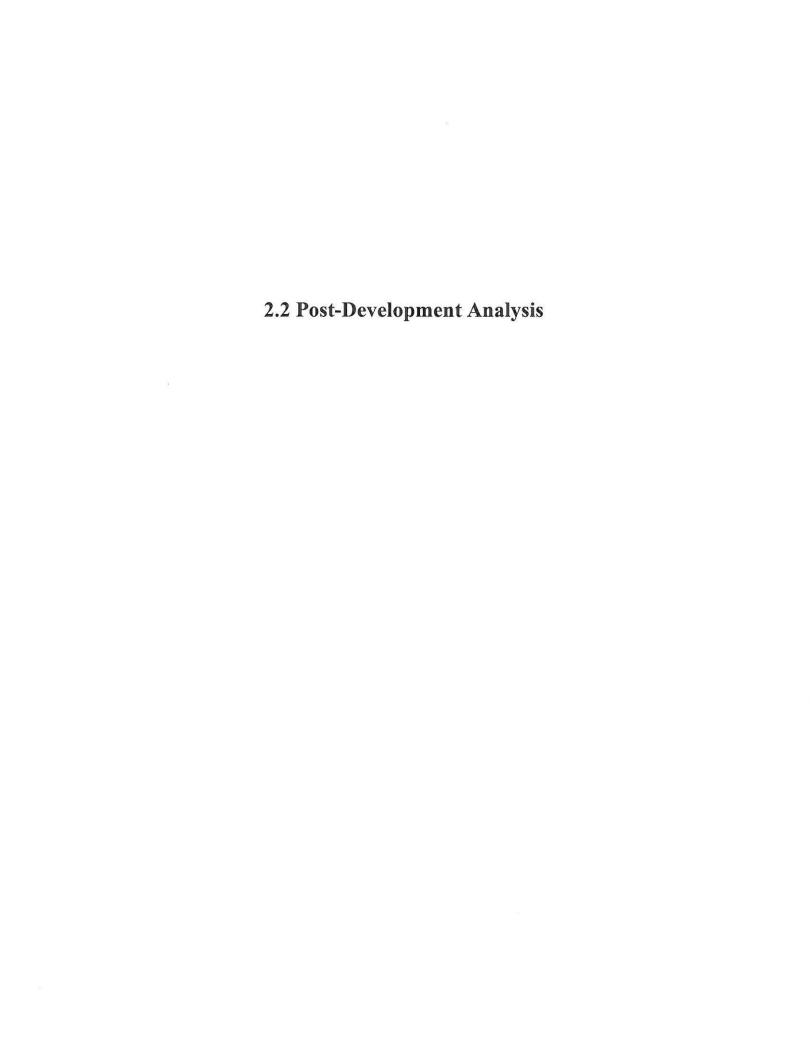
Inflow 9.280 MI

0.549 m³/s @ 14.47 hrs, Volume= 0.549 m³/s @ 14.47 hrs, Volume= 9.280 MI, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link Z: Drainage Point





2.2 Post-development Analysis

As in the pre-development analysis, the total watershed area is 816.84 hectares. Pre-development area boundaries remained virtually unchanged in the post-development analysis, and are similarly identified as Po1, Po2, Po3, and Po4. A smaller subcatchment exists within Po4, and has been identified in the model as Po4a.

Boundaries of drainage area Po1 are very similar to those of Pr1. External boundaries are entirely unchanged, while internal boundary with Po2 and Po4 are slightly modified by grading associated with the project. Twelve of the new turbines proposed for the project are contained within Po1. Additionally approximately 2300 meters of new road are proposed. Post construction all but a 5 meter gravel travelled way will be reclaimed. Runoff leaves this area at drainage analysis point W.

Drainage area Po2 is the same basic area as Pr2 with the southwestern boundary slightly modified by the addition of a proposed turbine and road. One proposed turbine and approximately 64 meters of roadway are contained within this drainage area. Runoff leaves this area at drainage analysis point X.

Drainage area Po3 is the same area as Pr3, with boundaries unchanged from the pre-development model. Two proposed turbines and approximately 536 meters of roadway are contained within this drainage area. Runoff leaves this area at drainage analysis point Y.

Drainage area Po4 is the same area as Pr4 with the southwestern boundary slightly modified by the addition of a proposed turbine and road. Three proposed turbines and approximately 371 meters of roadway are contained within this drainage area. The electrical substation, operations and maintenance area, and a laydown yard are located within Po4, draining to a sinkhole/pond. Drainage to this pond has been identified as a unique subcatchment, Po4a. Runoff leaves Po4 at drainage analysis point Z.

For more detailed information on post-developed areas, see attached drainage plans found in **Section 3** and the HydroCAD area listings found in **Section 2.2.2**. A pre- versus post-development comparison flow rate table for the 2, 10, and 50 year storm events can be found in **Table 1.3** in **Section 1.1.5**.

2.2.1 Post-Development 2, 10, and 50 - Year Storm

POST DRAINAGE

Type II 24-hr 2-Year Rainfall=85 mm Printed 1/31/2014

Prepared by Horizons Engineering, Inc. HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Page 1

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment Po1: Drainage Area Runoff Area=7,096,962.6 m² 1.02% Impervious Runoff Depth=0 mm Flow Length=3,573.8 m Tc=118.8 min CN=31 Runoff=0.000 m³/s 0.000 MI

Subcatchment Po2: Drainage Area

Runoff Area=164,097.8 m² 1.24% Impervious Runoff Depth=0 mm
Flow Length=581.1 m Tc=47.4 min CN=31 Runoff=0.000 m³/s 0.000 MI

Subcatchment Po3: Drainage Area Runoff Area=273,851.4 m² 3.62% Impervious Runoff Depth=0 mm Flow Length=1,033.9 m Tc=98.1 min CN=32 Runoff=0.000 m³/s 0.000 MI

Subcatchment Po4: Drainage Area Runoff Area=584,719.0 m² 2.90% Impervious Runoff Depth=0 mm Flow Length=2,751.8 m Tc=158.7 min CN=32 Runoff=0.000 m³/s 0.000 MI

Subcatchment Po4a: Drainage Area Runoff Area=48,731.9 m² 13.53% Impervious Runoff Depth>0 mm
Flow Length=480.2 m Slope=0.0200 m/m Tc=46.2 min CN=39 Runoff=0.000 m³/s 0.000 MI

Pond SH: Sinkhole/Pond Peak Elev=683.000 m Storage=0.1 m³ Inflow=0.000 m³/s 0.000 MI

Outflow=0.000 m3/s 0.000 MI

Link W: Drainage Point Inflow=0.000 m³/s 0.000 MI

Primary=0.000 m³/s 0.000 MI

Link X: Drainage Point Inflow=0.000 m³/s 0.000 MI

Primary=0.000 m³/s 0.000 MI

Link Y: Drainage Point Inflow=0.000 m³/s 0.000 MI

Primary=0.000 m3/s 0.000 MI

Link Z: Drainage Point Inflow=0.000 m³/s 0.000 MI

Primary=0.000 m3/s 0.000 MI

Total Runoff Area = 816.8363 ha Runoff Volume = 0.000 MI Average Runoff Depth = 0 mm 98.68% Pervious = 806.0445 ha 1.32% Impervious = 10.7918 ha

HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Page 2

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment Po1: Drainage Area Runoff Area=7,096,962.6 m² 1.02% Impervious Runoff Depth>14 mm Flow Length=3,573.8 m Tc=118.8 min CN=31 Runoff=6.667 m³/s 101.801 MI

Subcatchment Po2: Drainage Area Runoff Area=164,097.8 m² 1.24% Impervious Runoff Depth>16 mm Flow Length=581.1 m Tc=47.4 min CN=31 Runoff=0.249 m³/s 2.545 MI

Subcatchment Po3: Drainage Area Runoff Area=273,851.4 m² 3.62% Impervious Runoff Depth>17 mm
Flow Length=1,033.9 m Tc=98.1 min CN=32 Runoff=0.333 m³/s 4.596 MI

Subcatchment Po4: Drainage Area Runoff Area=584,719.0 m² 2.90% Impervious Runoff Depth>16 mm Flow Length=2,751.8 m Tc=158.7 min CN=32 Runoff=0.539 m³/s 9.115 MI

Subcatchment Po4a: Drainage Area Runoff Area=48,731.9 m² 13.53% Impervious Runoff Depth>34 mm Flow Length=480.2 m Slope=0.0200 m/m Tc=46.2 min CN=39 Runoff=0.254 m³/s 1.677 MI

Pond SH: Sinkhole/Pond Peak Elev=685.013 m Storage=1,451.5 m³ Inflow=0.254 m³/s 1.677 MI

Outflow=0.026 m3/s 0.229 MI

Link W: Drainage Point Inflow=6.667 m³/s 101.801 MI Primary=6.667 m³/s 101.801 MI

Link X: Drainage Point Inflow=0.249 m³/s 2.545 MI Primary=0.249 m³/s 2.545 MI

Link Y: Drainage Point Inflow=0.333 m³/s 4.596 MI Primary=0.333 m³/s 4.596 MI

Link Z: Drainage Point Inflow=0.539 m³/s 9.344 MI Primary=0.539 m³/s 9.344 MI

Total Runoff Area = 816.8363 ha Runoff Volume = 119.734 MI Average Runoff Depth = 15 mm 98.68% Pervious = 806.0445 ha 1.32% Impervious = 10.7918 ha

HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Page 3

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment Po1: Drainage Area Runoff Area=7,096,962.6 m² 1.02% Impervious Runoff Depth>93 mm Flow Length=3,573.8 m Tc=118.8 min CN=31 Runoff=59.832 m³/s 659.449 MI

Subcatchment Po2: Drainage Area Runoff Area=164,097.8 m² 1.24% Impervious Runoff Depth>97 mm Flow Length=581.1 m Tc=47.4 min CN=31 Runoff=2.748 m³/s 15.972 MI

Subcatchment Po3: Drainage Area Runoff Area=273,851.4 m² 3.62% Impervious Runoff Depth>100 mm Flow Length=1,033.9 m Tc=98.1 min CN=32 Runoff=2.878 m³/s 27.426 MI

Subcatchment Po4: Drainage Area Runoff Area=584,719.0 m² 2.90% Impervious Runoff Depth>96 mm Flow Length=2,751.8 m Tc=158.7 min CN=32 Runoff=4.216 m³/s 55.960 MI

Subcatchment Po4a: Drainage Area Runoff Area=48,731.9 m² 13.53% Impervious Runoff Depth>144 mm Flow Length=480.2 m Slope=0.0200 m/m Tc=46.2 min CN=39 Runoff=1.330 m³/s 7.024 MI

Pond SH: Sinkhole/Pond Peak Elev=685.196 m Storage=1,672.5 m³ Inflow=1.330 m³/s 7.024 MI

Outflow=1.292 m3/s 5.555 MI

Link W: Drainage PointInflow=59.832 m³/s 659.449 MI
Primary=59.832 m³/s 659.449 MI

Link X: Drainage Point Inflow=2.748 m³/s 15.972 MI

Primary=2.748 m3/s 15.972 MI

Link Y: Drainage Point Inflow=2.878 m³/s 27.426 MI

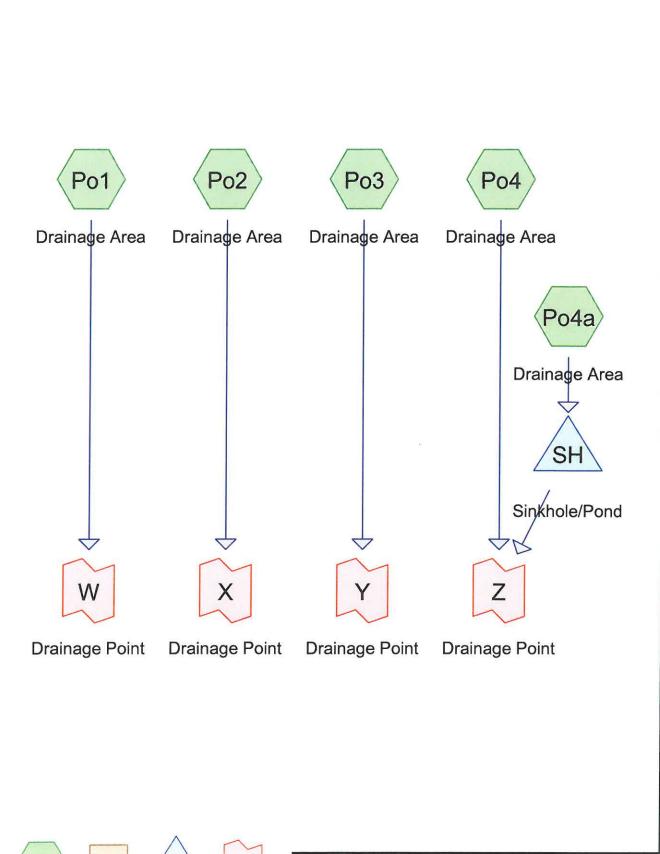
Primary=2.878 m³/s 27.426 MI

Link Z: Drainage Point Inflow=4.445 m³/s 61.515 MI

Primary=4.445 m³/s 61.515 MI

Total Runoff Area = 816.8363 ha Runoff Volume = 765.830 MI Average Runoff Depth = 94 mm 98.68% Pervious = 806.0445 ha 1.32% Impervious = 10.7918 ha

2.2.2 Post-Development Full Summary Diagram 10 - Year Storm Event











POST_DRAINAGE
Prepared by Horizons Engineering, Inc.
HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Printed 1/31/2014 Page 2

Area Listing (all nodes)

Area	CN	Description
(hectares)		(subcatchment-numbers)
234.0845	30	Meadow, non-grazed, HSG A (Po1, Po2, Po3, Po4, Po4a)
16.8781	30	Meadow, non-grazed, PROJECT, HSG A (Po1, Po2, Po3, Po4, Po4a)
555.0460	30	Woods, Good, HSG A (Po1, Po2, Po3, Po4, Po4a)
3.5885	98	Gravel-Project Road/Pads, 99% imp, HSG A (Po1, Po2, Po3, Po4, Po4a)
0.0669	98	Roofs, HSG A (Po1, Po2, Po3, Po4, Po4a)
1.0845	98	xBUILDING (Po1)
6.0879	98	xROAD (Po1, Po2, Po3, Po4)
816.8363	31	TOTAL AREA

POST_DRAINAGE
Prepared by Horizons Engineering, Inc.
HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Printed 1/31/2014 Page 3

Soil Listing (all nodes)

Area	Soil	Subcatchment
(hectares)	Group	Numbers
809.6640	HSG A	Po1, Po2, Po3, Po4, Po4a
0.0000	HSG B	
0.0000	HSG C	
0.0000	HSG D	
7.1723	Other	Po1, Po2, Po3, Po4
816.8363		TOTAL AREA

POST_DRAINAGE
Prepared by Horizons Engineering, Inc.
HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Printed 1/31/2014

Page 4

Pipe Listing (all nodes)

Line#	Node	In-Invert	Out-Invert	Length	Slope	n	Diam/Width	Height	Fill
	Number	(meters)	(meters)	(meters)	(m/m)		(mm)	(mm)	(mm)
1	Po3	0.000	0.000	45.00	0.0889	0.013	457	0	0

POST DRAINAGE

Type II 24-hr 10-Year Rainfall=228 mm Printed 1/31/2014

Prepared by Horizons Engineering, Inc.
HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Page 5

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment Po1: Drainage Area Runoff Area=7,096,962.6 m² 1.02% Impervious Runoff Depth>14 mm Flow Length=3,573.8 m Tc=118.8 min CN=31 Runoff=6.667 m³/s 101.801 MI

Subcatchment Po2: Drainage Area Runoff Area=164,097.8 m² 1.24% Impervious Runoff Depth>16 mm Flow Length=581.1 m Tc=47.4 min CN=31 Runoff=0.249 m³/s 2.545 MI

Subcatchment Po3: Drainage Area Runoff Area=273,851.4 m² 3.62% Impervious Runoff Depth>17 mm Flow Length=1,033.9 m Tc=98.1 min CN=32 Runoff=0.333 m³/s 4.596 MI

Subcatchment Po4: Drainage Area Runoff Area=584,719.0 m² 2.90% Impervious Runoff Depth>16 mm Flow Length=2,751.8 m Tc=158.7 min CN=32 Runoff=0.539 m³/s 9.115 MI

Subcatchment Po4a: Drainage Area

Runoff Area=48,731.9 m² 13.53% Impervious Runoff Depth>34 mm
Flow Length=480.2 m Slope=0.0200 m/m Tc=46.2 min CN=39 Runoff=0.254 m³/s 1.677 MI

Pond SH: Sinkhole/Pond Peak Elev=685.013 m Storage=1,451.5 m³ Inflow=0.254 m³/s 1.677 MI

Outflow=0.026 m3/s 0.229 MI

Link W: Drainage Point Inflow=6.667 m³/s 101.801 MI
Primary=6.667 m³/s 101.801 MI

Link X: Drainage Point Inflow=0.249 m³/s 2.545 MI
Primary=0.249 m³/s 2.545 MI

Link Y: Drainage Point Inflow=0.333 m³/s 4.596 MI Primary=0.333 m³/s 4.596 MI

Link Z: Drainage Point Inflow=0.539 m³/s 9.344 MI Primary=0.539 m³/s 9.344 MI

Total Runoff Area = 816.8363 ha Runoff Volume = 119.734 MI Average Runoff Depth = 15 mm 98.68% Pervious = 806.0445 ha 1.32% Impervious = 10.7918 ha

Printed 1/31/2014 Page 6

Summary for Subcatchment Po1: Drainage Area

6.667 m³/s @ 13.93 hrs, Volume= Runoff

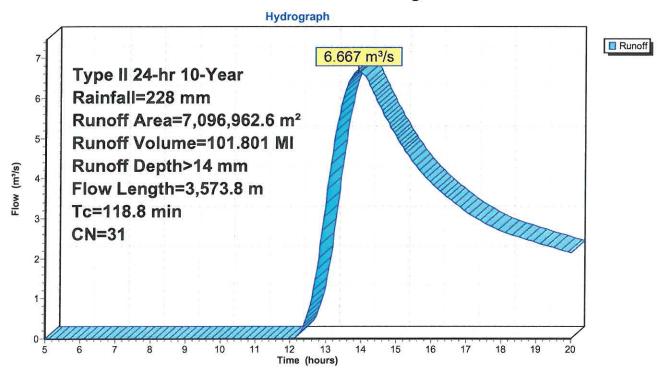
101.801 MI, Depth> 14 mm

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=228 mm

	Α	rea (m²)	CN [Description							
	5,42	20,293.8	30 \	Woods, Good, HSG A							
4	· 4	4,666.0	98 >	xROAD							
4	· 1	0,844.6	98 >	BUILDING							
	1,51	3,866.9	30	Meadow, non-grazed, HSG A							
3	1	6,940.2				ads, 99% imp, HSG A					
	9	0,214.2				PROJECT, HSG A					
		136.9	98 F	Roofs, HSG	Α		_				
	7,09	6,962.6	31 V	Veighted Av	verage						
	7,02	4,544.3	ç	8.98% Pen	vious Area						
	7	2,418.3	1	.02% Impe	rvious Area						
	Тс	Length	Slope		Capacity	Description					
	(min)	(meters)	(m/m)	(m/sec)	(m³/s)		_				
	28.6	31.0	0.0100	0.02		Sheet Flow, Po1-A					
						Grass: Bermuda n= 0.410 P2= 85 mm					
	19.7	581.0	0.1048	0.49		Shallow Concentrated Flow, Po1-B					
					Woodland Kv= 1.52 m/s						
	19.5	697.5	0.0781	0.60		Shallow Concentrated Flow, Po1-C					
		141102 Decreases (1704-173	0.00 PR. 1000 W2000			Short Grass Pasture Kv= 2.13 m/s					
	11.6	767.9	0.5307	1.11		Shallow Concentrated Flow, Po1-D					
	NEST 200	92.752.722 P25		12112112		Woodland Kv= 1.52 m/s					
	7.2	355.3	0.1511	0.83		Shallow Concentrated Flow, Po1-E					
						Short Grass Pasture Kv= 2.13 m/s					
	32.2	1,141.1	0.1514	0.59		Shallow Concentrated Flow, Po1-F					
2						Woodland Kv= 1.52 m/s	_				
	118.8	3,573.8	Total								

Page 7

Subcatchment Po1: Drainage Area



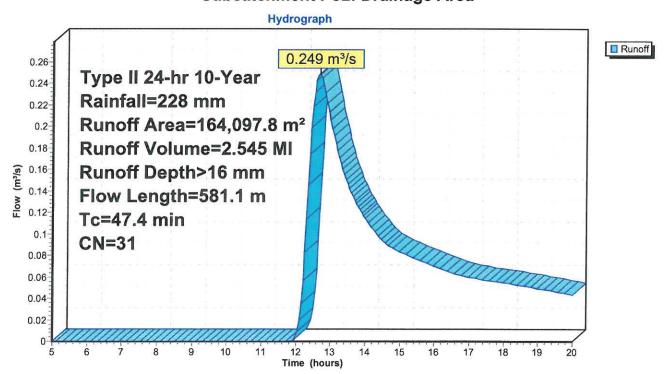
Summary for Subcatchment Po2: Drainage Area

Runoff = 0.249 m³/s @ 12.69 hrs, Volume= 2.545 MI, Depth> 16 mm

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=228 mm

	Α	rea (m²)	CN	Des	cription							
0.77	4	14,734.9	30	Woo	Woods, Good, HSG A							
*		1,336.4	98	xRO	ROAD							
*		0.0	98	xBU	ILDING							
	11	4,185.0	30	Mea	dow, no	n-grazed, F	ISG A					
*		687.5	98	Grav	el-Proje	ct Road/Pa	ids, 99% imp, HSG A					
		3,141.5	30	Mea	dow, no	n-grazed, F	PROJECT, HSG A					
		12.5	98	Roo	fs, HSG	Α						
	16	64,097.8	31	Weig	ghted Av	erage						
	162,068.3					ious Area						
		2,029.5		1.24	% Imper	rvious Area						
	Tc	Length	Slop	oe ∖	/elocity	Capacity	Description					
	(min)	(meters)	(m/r	n) (m/sec)	(m³/s)	Andrewson and Control of the Control					
	21.3	31.0	0.020	00	0.02		Sheet Flow, Po2-A					
	100000000000000000000000000000000000000		8705 SEATTER	100EU			Woods: Light underbrush n= 0.400 P2= 85 mm					
	26.1	550.1	0.027	73	0.35		Shallow Concentrated Flow, Po2-B					
							Short Grass Pasture Kv= 2.13 m/s					
	47.4	581.1	Total									

Subcatchment Po2: Drainage Area



Page 9

Summary for Subcatchment Po3: Drainage Area

0.333 m³/s @ 13.50 hrs, Volume= Runoff

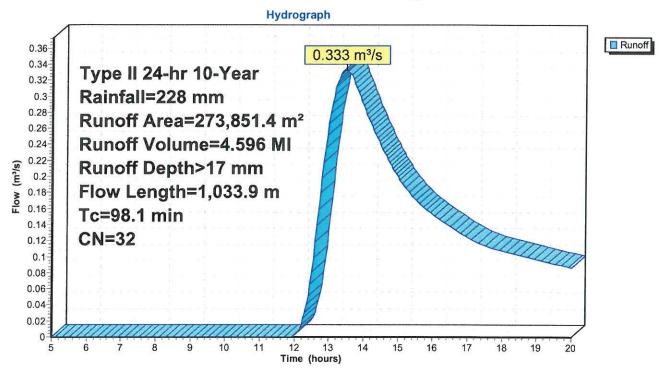
4.596 MI, Depth> 17 mm

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=228 mm

	Α	rea (m²)	CN D	escription								
		635.7	30 V	Voods, Goo	d, HSG A							
*		5,472.0	98 x	ROAD								
*		0.0	98 x	BUILDING								
	24	4,591.7	30 N	Meadow, non-grazed, HSG A								
*		4,452.3	98 G	ravel-Proje	ct Road/Pa	ads, 99% imp, HSG A						
	1	8,674.8				PROJECT, HSG A						
		24.9	98 R	oofs, HSG	Α							
	27	3,851.4	32 V	Veighted Av	/erage							
	26	3,946.7	9	6.38% Pen	vious Area							
		9,904.7	3	.62% Impe	rvious Area							
	-		200	2 2 2 1201	_							
8	Tc	Length	Slope		Capacity	Description						
	nin)	(meters)	(m/m)		(m³/s)							
2	28.6	31.0	0.0100	0.02		Sheet Flow, Po3-A						
	0.0		2000	12 12 1		Grass: Bermuda n= 0.410 P2= 85 mm						
4	13.6	618.1	0.0123	0.24		Shallow Concentrated Flow, Po3-B						
						Short Grass Pasture Kv= 2.13 m/s						
	0.1	45.0	0.0889	5.40	0.8858	Pipe Channel, Po3-C						
						457 mm Round Area= 0.16 m ² Perim= 1.44 m r= 0.11 m						
_						n= 0.013 Corrugated PE, smooth interior						
2	25.8	339.8	0.0106	0.22		Shallow Concentrated Flow, Po3-D						
		- West - Selection Co. 1997	history as a			Short Grass Pasture Kv= 2.13 m/s						
9	8.1	1,033.9	Total									

Page 10

Subcatchment Po3: Drainage Area



HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Page 11

Summary for Subcatchment Po4: Drainage Area

Runoff

0.53

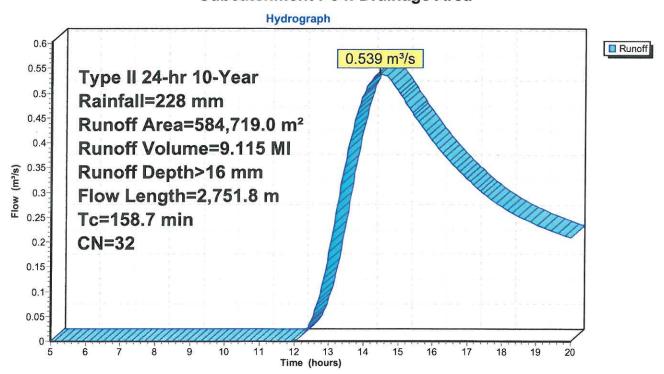
0.539 m³/s @ 14.47 hrs, Volume=

9.115 MI, Depth> 16 mm

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=228 mm

	Α	rea (m²)	CN	Descriptio	n							
	8	31,838.2	30	Woods, G	ood, HSG A							
*		9,404.2	98	xROAD								
*		0.0	98	xBUILDIN	G							
	45	5,264.0	30	Meadow,	non-grazed,	HSG A						
*		7,606.7	98	Gravel-Pro	ject Road/F	Pads, 99% imp, HSG A						
	3	0,568.6	30	Meadow,	non-grazed,	PROJECT, HSG A						
1		37.3	98	Roofs, HS	G A							
	58	4,719.0	32	Weighted	Average							
	567,746.9			97.10% P	ervious Area	Í						
	1	6,972.1		2.90% Imp	ervious Are	a						
	Tc Length		Slop	pe Velocit	y Capacity							
	(min)	(meters)	(m/r	n) (m/sec	c) (m³/s)							
	12.5	31.0	0.075	57 0.0	4	Sheet Flow, Po4-A						
						Woods: Light underbrush n= 0.400 P2= 85 mm						
1	146.2	2,720.8	0.02	12 0.3	1	Shallow Concentrated Flow, Po4-B						
		8				Short Grass Pasture Kv= 2.13 m/s						
1	158.7	2.751.8	Total									

Subcatchment Po4: Drainage Area



Summary for Subcatchment Po4a: Drainage Area

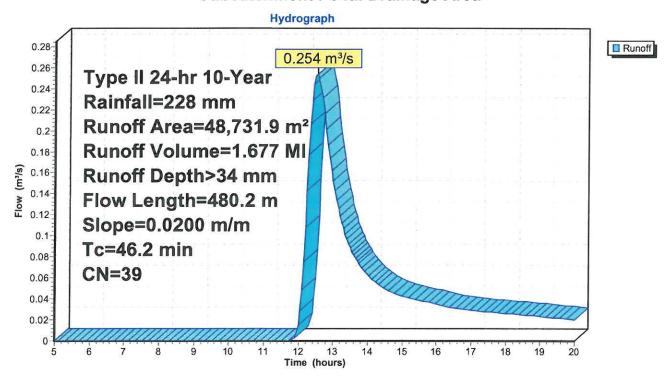
Runoff = 0.254 m³/s @ 12.54 hrs, Volume=

1.677 MI, Depth> 34 mm

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 10-Year Rainfall=228 mm

	Α	rea (m²)	CN	De	escription			
		2,957.6	30	W	oods, Goo	d, HSG A		
*		0.0	98	хF	ROAD			
*		0.0	98	χE	BUILDING			
	1	2,937.3	30	Me	eadow, no	n-grazed, F	HSG A	
*		6,198.0	98	Gr	avel-Proje	ct Road/Pa	ads, 99% imp, HSG A	
	2	26,181.5	30	Me	eadow, no	n-grazed, F	PROJECT, HSG A	
		457.5	98	Ro	ofs, HSG	A		
	4	18,731.9	39	W	eighted Av	erage		
	4		86.47% Pervious Area					
		6,593.5		13	.53% Impe	ervious Are	a	
	Tc	Length	Slo	pe	Velocity	Capacity	Description	
_	(min)	(meters)	(m/	m)_	(m/sec)	(m³/s)		
	21.3 31.0		0.02	00	0.02		Sheet Flow, Po4a-A	
							Woods: Light underbrush n= 0.400 P2= 85 mm	
	24.9	449.2	0.02	00	0.30		Shallow Concentrated Flow, Po4a-B	
							Short Grass Pasture Kv= 2.13 m/s	
_	46.2	480.2	Tota	1				

Subcatchment Po4a: Drainage Area



POST DRAINAGE

Prepared by Horizons Engineering, Inc.

HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Page 13

Summary for Pond SH: Sinkhole/Pond

Inflow Area = 4.8732 ha, 13.53% Impervious, Inflow Depth > 34 mm for 10-Year event

Inflow = $0.254 \,\mathrm{m}^3/\mathrm{s}$ @ $12.54 \,\mathrm{hrs}$, Volume= $1.677 \,\mathrm{MI}$

Outflow = 0.026 m³/s @ 17.92 hrs, Volume= 0.229 Ml, Atten= 90%, Lag= 322.5 min

Primary = 0.026 m³/s @ 17.92 hrs, Volume= 0.229 MI

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Peak Elev= 685.013 m @ 17.92 hrs Surf.Area= 1,158.2 m² Storage= 1,451.5 m³

Plug-Flow detention time= 379.8 min calculated for 0.228 MI (14% of inflow)

Center-of-Mass det. time= 256.0 min (1,119.3 - 863.3)

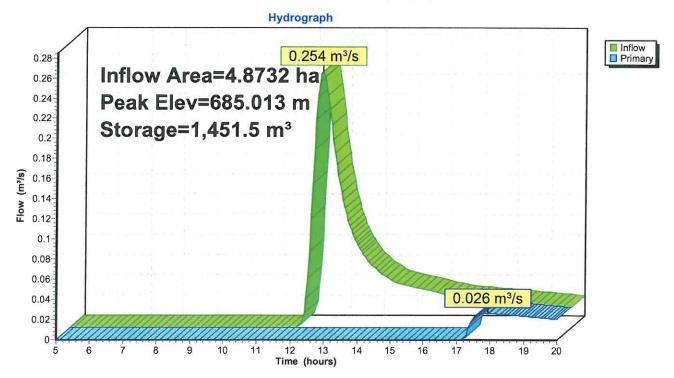
Volume	<u>In</u>	vert Avail	l.Storage	Storage	Description	
#1 683.00		0 m 2,	075.0 m³	Custom	ismatic)Listed below (Recalc)	
Elevatio _(meters	76 G 7	Surf.Area (sq-meters)	Inc (cubic-m	:.Store leters)	Cum.Store (cubic-meters)	
683.00	0	336.0		0.0	0.0	
684.00	0	693.0		514.5	514.5	
685.00	0	1,152.0		922.5	1,437.0	
685.50	0	1,400.0		638.0	2,075.0	
Device	Routing	j Inve	ert Outlet	t Devices		
#1	Primary	685.000	Head	(meters)	0.061 0.122 0.	th Broad-Crested Rectangular Weir .183 0.244 0.305 0.366 0.427
			Coef.	(Metric)		14 1.067 1.219 1.372 1.524 1.676 1.48 1.48 1.47 1.47 1.46 1.46 50 1.51 1.55

Primary OutFlow Max=0.019 m³/s @ 17.92 hrs HW=685.013 m (Free Discharge) 1=Broad-Crested Rectangular Weir (Weir Controls 0.019 m³/s @ 0.15 m/s)

Prepared by Horizons Engineering, Inc. HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Page 14

Pond SH: Sinkhole/Pond



Page 15

Summary for Link W: Drainage Point

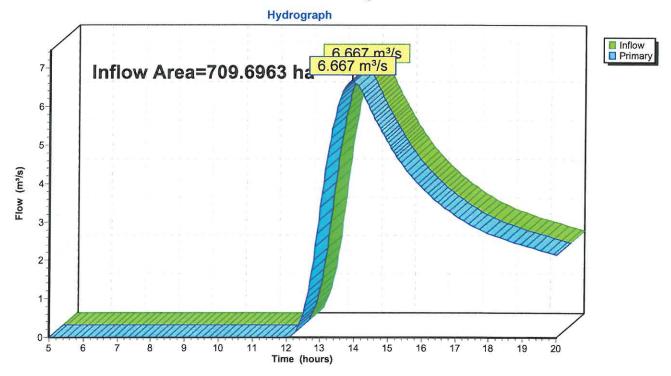
Inflow Area = 709.6963 ha, 1.02% Impervious, Inflow Depth > 14 mm for 10-Year event

Inflow = $6.667 \,\mathrm{m}^3/\mathrm{s}$ @ $13.93 \,\mathrm{hrs}$, Volume= $101.801 \,\mathrm{MI}$

Primary = 6.667 m³/s @ 13.93 hrs, Volume= 101.801 Ml, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link W: Drainage Point



HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC

Page 16

Summary for Link X: Drainage Point

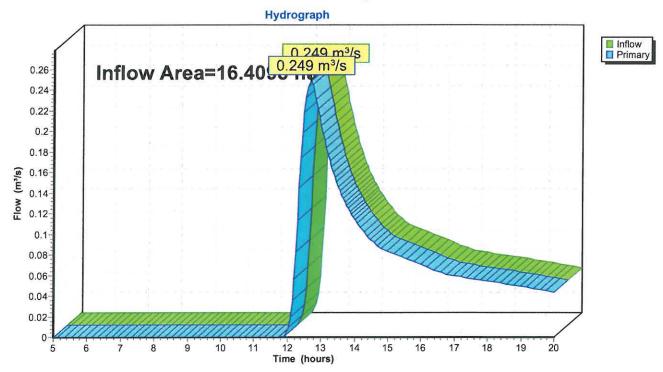
1.24% Impervious, Inflow Depth > 16 mm for 10-Year event Inflow Area = 16.4098 ha,

0.249 m³/s @ 12.69 hrs, Volume= 2.545 MI Inflow

0.249 m³/s @ 12.69 hrs, Volume= 2.545 MI, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link X: Drainage Point



Prepared by Horizons Engineering, Inc. HydroCAD® 9.10 s/n 04721 © 2010 HydroCAD Software Solutions LLC Printed 1/31/2014

Page 17

Summary for Link Y: Drainage Point

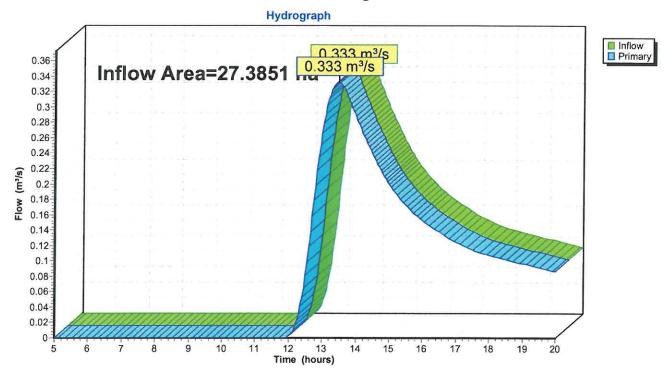
Inflow Area = 27.3851 ha, 3.62% Impervious, Inflow Depth > 17 mm for 10-Year event

Inflow = $0.333 \text{ m}^3/\text{s}$ @ 13.50 hrs, Volume= 4.596 MI

Primary = 0.333 m³/s @ 13.50 hrs, Volume= 4.596 MI, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link Y: Drainage Point



Printed 1/31/2014 Page 18

Summary for Link Z: Drainage Point

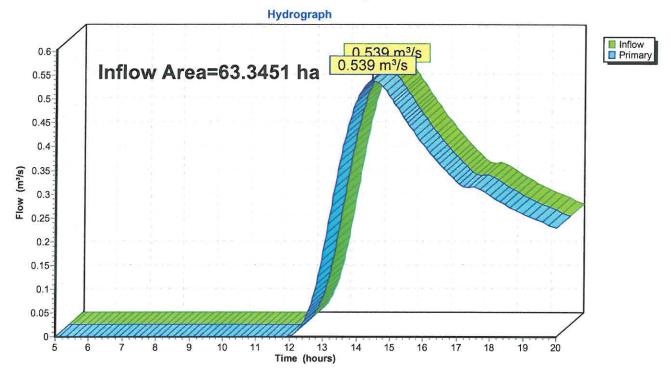
Inflow Area = 63.3451 ha, 3.72% Impervious, Inflow Depth > 15 mm for 10-Year event

Inflow = $0.539 \,\text{m}^3/\text{s}$ @ $14.47 \,\text{hrs}$, Volume= $9.344 \,\text{MI}$

Primary = 0.539 m³/s @ 14.47 hrs, Volume= 9.344 MI, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link Z: Drainage Point



2.3 References
Preparer's Certification
Reviewer's Certification

PREPARER'S CERTIFICATION

Christoph J. Himle Prepared by Chris Hernick

REVIEWER'S CERTIFICATION

Reviewed by Steve LaFrance, P.Eng.





This Section Left Intentionally Blank (Plans Bound Separately)

3.2 Pre- & Post-Development Drainage Area Plans